NEW FREE-STANDING GAZEBO

FRAMING AND FOUNDATION DESIGN

240 E SUMMIT AVENUE

SAN ANTONIO, TEXAS 78212

CLIENT:

RICK GONZALEZ

DRAWING INDEX:

Darr Engineering
FOUNDATION AND FRAMING CONSULTANTS

TEXAS REGISTERED
ENGINEERING FIRM
F-24336

933 N FLORES STREET
SAN ANTONIO, TEXAS 78212

San Antonio: (210) 300-6767

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240 E SUMMIT AVENUE
SAN ANTONIO, TEXAS 78212
FOUNDATION AND FRAMING DESIGN

STATE OF TEXAS
BRANDON T. DARR
142173
LICENSED
PROFESSIONAL ENGINEER

2/10/2026

Project #: 25-782

Date: 2/10/2026

Title Sheet

Sheet No. S0.0

STRUCTURAL NOTES

GENERAL REQUIREMENTS

The general contractor and all subcontractors performing the work outlined in these drawings shall familiarize themselves with the drawings, details, and notes outlined in this document. Questions and concerns shall be relayed to the Engineer.

Codes

2021 International Building Code

ASCE 7-16 Minimum Design Loads for Buildings and Other Structures

All referenced codes and standards shall include all amendments and addenda in force at the dates of the contract documents. Where conflict exists among the various referenced publications and the contract documents, contact the Engineer.

Structural Design Criteria

Design Loads

Dead loads: Self-weight of the structure

Live loads: Roof 20 psf

Wind Loads:

Design Wind Speed V/B - 115 mph

Risk Category - II

Exposure Class - B

Main Wind-Force Resisting (MWFRS):

Existing Conditions

Field verify all existing structural dimensions and conditions. Dimensions shown on the plans are approximate. Field measurements will be required to complete the work. Contractor shall obtain all field measurements as necessary to coordinate with and match new construction to existing conditions.

Some information on these drawings regarding existing features is necessarily conjectural due to unknown conditions at the time of preparation. If conditions exist that differ from the drawings or are not adequately detailed, inform the Engineer and additional details or interpretation will be provided. Do not proceed without verification from the Engineer.

Substitutions

All requests for substitutions of materials or details shown in the contract documents shall be submitted for approval prior to their use. The Contractor shall allocate a minimum of 5 business days for response/approval of any proposed substitutions or changes.

For any substitutions causing or requiring changes to the structure or the structural design, the Contractor shall bear the responsibility for all consequent additional design and coordination.

Coordination

Compare structural drawings with architectural, mechanical, and civil drawings and report any discrepancy to the Engineer prior to fabrication or installation of structural members.

The details or notes designated as "typical" or “typ” apply generally to the drawings in all areas where conditions are similar to those described as typical.

Temporary Bracing, Falsework, and Formwork

The design, construction, and safety of all temporary supports, such as guys, braces, falsework, formwork, shores, and bracing required for the execution of the contract, are not included in the drawings and shall be the responsibility of the Contractor.

Where a design has been provided by the Engineer, the Contractor retains responsibility for the construction and safety of these temporary supports. Contact the Engineer immediately if there is increased cracking, movement, or apparent instability.

Submittals

The following items shall be submitted to the Engineer for review. Refer to the Specifications for additional submittal items. Submit under provisions of the Contract Documents. Work associated with these items shall not commence until

Engineer Review of Construction

On-site review of construction by the Engineer is required at the following stages:

Foundation - Prior to placement of concrete.

Steel Framing - When complete

Close-out letters will not be issued without satisfactory review by the Engineer of the above stages/items.

Contact Engineer at least 72 hours prior to the expected time the review is needed.

Obtain Engineer's review of any unforeseen or differing structural conditions.

STRUCTURAL SPECIFICATION NOTES AND DATA

Site Drainage

Slope grade to drain away from building in all areas.

Roof drains and downspouts shall discharge away from foundation, and storm water shall be routed away from building.

Foundations & Excavation

Foundation design is based on the *Bexar County Soil Survey* provided by the NRCS Web Soil Survey.

Design soil bearing pressure is 1500 psf based on the “Presumptive Load-Bearing Values” given in the *International Building Code* Section 1806.

Excavations for footings shall be neat. Place concrete immediately after excavation and inspection. All water accumulation in footing shall be pumped out and allowed to dry for a minimum of 48 hours prior to placement of concrete.

Do not disturb material below the bottom of footing grade. Do not backfill to compensate for excavation that has extended below grade. If excavation occurs below the proposed footing grade, fill the area with concrete at the time the footing is placed.

If voids or pockets of loose, soft or compressible soil, deleterious materials, or organic matter are found in the bearing stratum, notify the Engineer immediately. Do not place the foundation until the Engineer has inspected the excavation and authorized changes have been made to provide a uniform bearing condition.

Cast-In-Place Concrete

Structural concrete shall comply with *Specifications for Structural Concrete For Buildings* (ACI 301) and *Manual of Standard Practice, Concrete Reinforcing Steel Institute* (CRSI).

Unless noted otherwise, cast-in-place concrete shall meet the following 28 day strength requirements:

Footings: 3,000 psi

Structural Steel

Structural steel shall comply with *Code Of Standard Practice, and Specification For The Design, Fabrication, And Erection Of Structural Steel For Buildings*, American Institute Of Steel Construction.

Splicing of structural steel members is prohibited without the approval of the Structural Engineer as to location and type. Any member having a splice not shown and detailed on the shop drawings will be rejected.

Burning of holes in structural steel is prohibited. Any member with burned holes shall be replaced.

Structural shapes and plates shall conform to the following, unless noted otherwise on the drawings:

Wideflange beams & columns: ASTM A992

Tubular members (HSS): ASTM A500, grade B

Other shapes & plates: ASTM A36

Bolts shall be ASTM A325 with ASTM A563 nuts and ASTM F436 washers.

Anchor bolts shall be ASTM F1554 Grade 55 minimum unless otherwise noted. All anchor bolts shall be furnished with and installed with large F844 standard washers.

Anchor bolts, nuts and washers shall be hot-dipped galvanized.

All column base plates shall be grouted immediately after the frame erection is completed and plumbed, and prior to pouring the slabs for floors or roofs.

Unless otherwise noted, connections are to be shop-welded and field-bolted.

All welding shall be done in accordance with A.W.S. Code by welders certified for the type of weld required.

Welding electrodes shall be 70 ksi low hydrogen electrodes, unless noted otherwise.

All miscellaneous welds (field or shop) shall be minimum size fillet all around in accordance with A.I.S.C. welding of continuous members. They shall be a minimum of 2-inches of 3/16-inch fillet stitch welds at 12-inches on center, staggered each side, unless otherwise noted.

All welds shall be de-slagged and cleaned prior to painting or galvanizing.

Anchors

Adhesive Anchors shall be equal to Hilti HIT HY-270 for Masonry and Hilti HY-200 for Solid Concrete. Screen tube required in hollow masonry. Field sample required to demonstrate performance.

Anchors shall be installed in accordance with the manufacturer's recommendations and the manufacturer's current ICBO report for the anchor. If a conflict exists between these referenced documents, the most stringent requirement shall govern.

Use the drill bit type and size recommended by the anchor manufacturer. Holes shall be drilled in a continuous operation, avoiding frequent removal of the drill from the hole. Holes shall not be enlarged or redirected anywhere along their length. After drilling, all dust and other foreign matter shall be blown out of the hole with compressed air.

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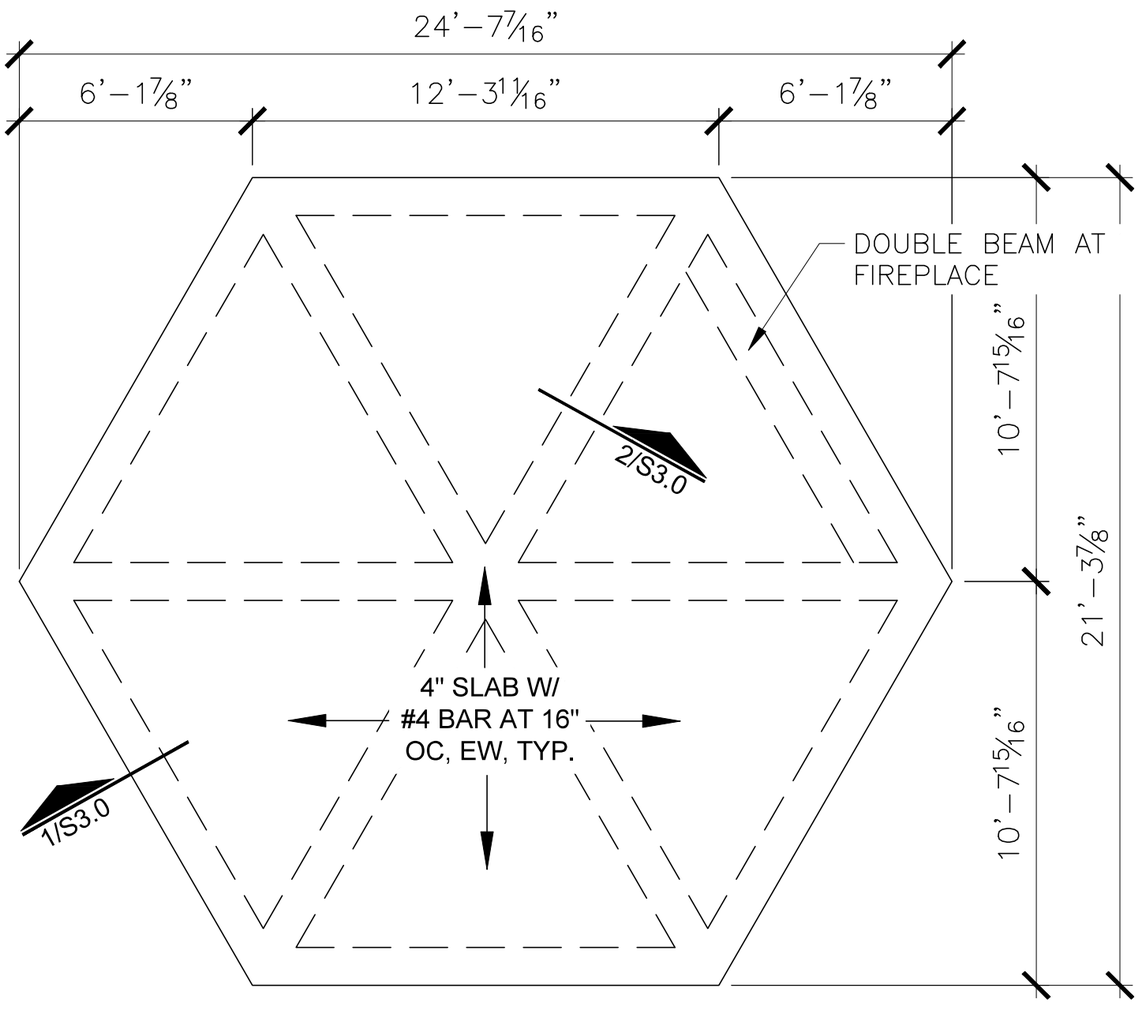
Notes

Sheet No. S1.0



GAZEBO SLAB FOUNDATION PLAN

¼" = 1'–0"

FOUNDATION NOTES

1. Refer to the General Notes on S1.0 for general information regarding the complete scope of work.
2. Verify all dimensions as identified on the plans with the Architectural drawings. Do not exceed the spans identified on the framing plan. All dimensions are approximate.
3. Excavate all footings to the minimum depth and width as identified on the details. Ensure footing holes are free of debris. Do not backfill holes which have been over excavated.
4. Verify all minimum clearances around steel reinforcement (rebar). Use rebar chairs or precast concrete blocks to provide minimum bottom clearance. Before placing rebar, ensure all rebar is clean of debris and free of rust.
5. Any conduit or pipes penetrating a footing must be sleeved with PVC pipe with a minimum thickness of standard schedule 40. A minimum clear distance of 1.5" must be retained around the conduit or pipe and the interior of the PVC sleeve. Tape ends of sleeve to prevent debris from entering the sleeve before or during placement of concrete.
6. Do not place concrete until the excavations and steel reinforcement have been reviewed by a licensed structural engineer. All deficiencies found during the inspection must be remediated, reviewed, and approved by the structural engineer prior to placing concrete.
7. Upon completion of the new footings, rake the soils around the concrete footings to allow for drainage around the individual footings.

FRAMING NOTES

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3. Ensure all connections (ex. joist to beam, column to beam) are flush. Gaps between members at bearing surfaces is not acceptable.
4. Fully weld all connections as identified on the structural plans. Ensure welds are clean and free of slag. Prime and paint all welds.
5. Do not allow penetrations into existing joists, rafters, or beams unless it meets the guidance established by a licensed structural engineer. Sizes and locations of penetrations must be approved by the engineer.
6. Call for an inspection by a licensed structural engineer when the framing is complete. All deficiencies found during the inspection must be remediated, reviewed, and approved by the structural engineer.

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Foundation and Framing Plan

Sheet No. S2.0

FOUNDATION NOTES

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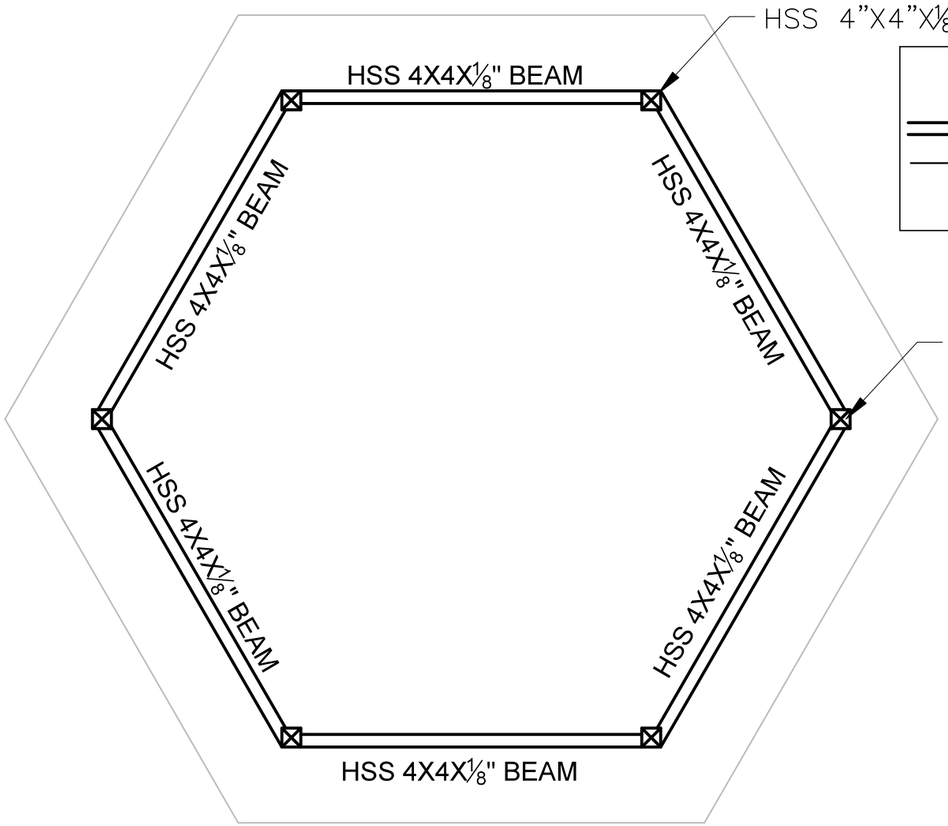
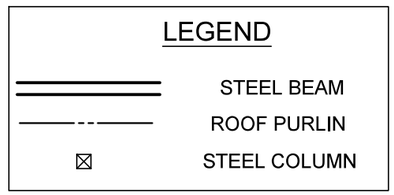
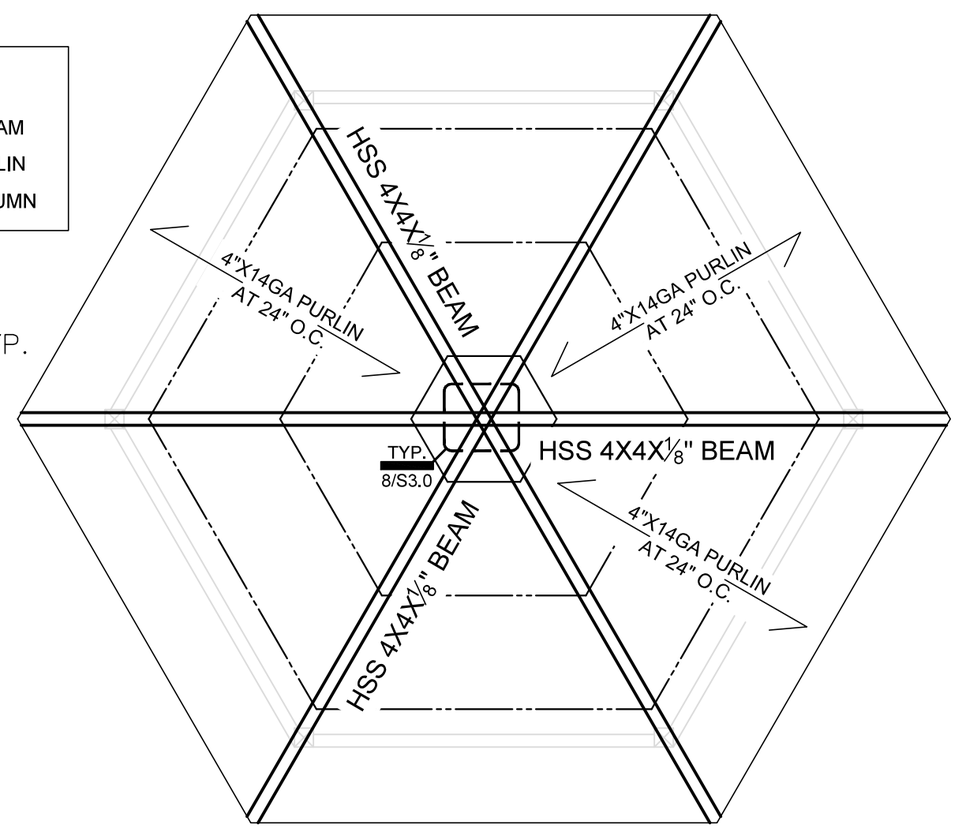


GAZEBO COLUMN FRAMING PLAN

¼" = 1'–0"





GAZEBO ROOF FRAMING PLAN

¼" = 1'–0"

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Foundation and Framing Plan

Sheet No. S2.1

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FOUNDATION KEYNOTES

1.10 - Slab thickness and reinforcement as per foundation plan. Refer to foundation plan on S2.0.

1.11 - (2) - #6 Continuous top and bottom reinforcement. Support bottom reinforcement with rebar chairs or blocking to maintain a minimum 3" clearance to the bottom of the grade beam.

1.11A - (2) - #4 Continuous bars at 12 inches on-center vertically. Place when overall beam height exceeds 36 inches.

1.12 - #3 Stirrups at 16" on-center along the length of the beam.

1.20 - Minimum 6" Compacted Select Fill, see general notes for additional information regarding type and quality of fill.

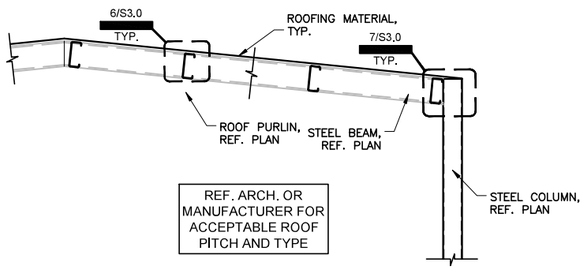
1.20A - Compacted Native Soil

1.20B - Continuous Lithic Rock

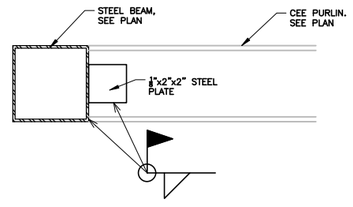
1.21 - Exterior beam must penetrate a minimum of 24" into the native soil (not including fill). Do not allow the bottom of the beam to bear on fill. Adjust overall beam height to allow for a minimum 24" penetration into the native soil while meeting the overall beam height.

1.21A - Interior beam must penetrate a minimum of 18" into the native soil (not including fill). Do not allow the bottom of the beam to bear on fill. Adjust overall beam height to allow for a minimum 18" penetration into the native soil.

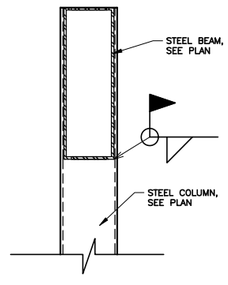
1.22 - 10 Mil Vapor Barrier, typ. Tape all seams and holes. Vapor barrier shall extend across the entirety of the slab. Bottom of grade beams shall not have a vapor barrier.



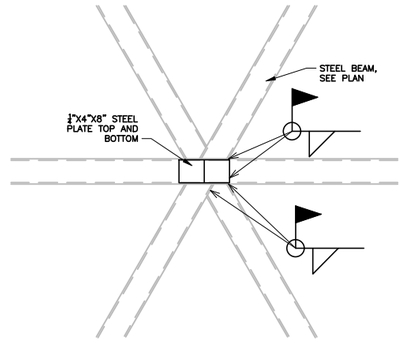
5 TYPICAL CARPORT ROOF SECTION
N.T.S.



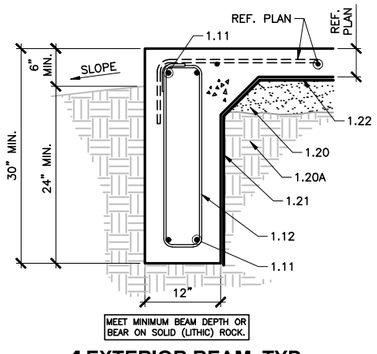
6 BEAM TO PURLIN CONNECTION, TYP
½" = 1'-0"



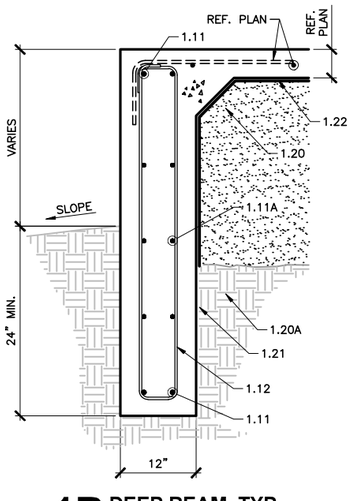
7 BEAM TO COLUMN CONNECTION, TYP
½" = 1'-0"



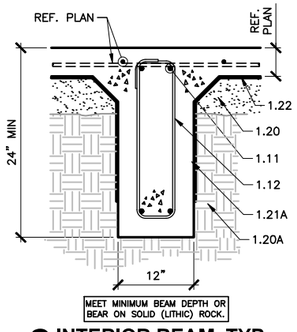
8 TYPICAL ROOF BEAM SECTION
N.T.S.



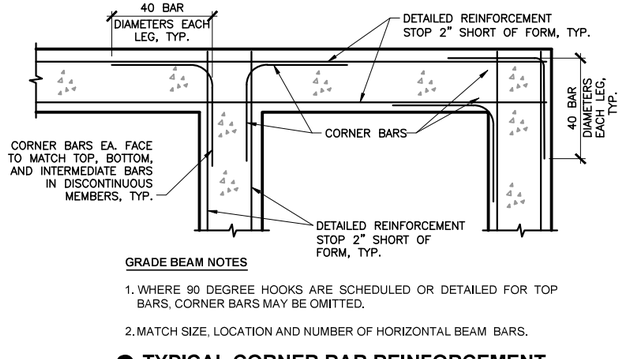
1 EXTERIOR BEAM, TYP.
1/2" = 1'-0"



1B DEEP BEAM, TYP.
1/2" = 1'-0"



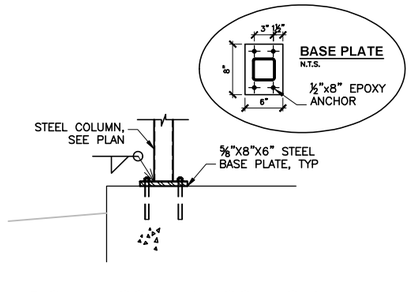
2 INTERIOR BEAM, TYP.
1/2" = 1'-0"



GRADE BEAM NOTES

1. WHERE 90 DEGREE HOOKS ARE SCHEDULED OR DETAILED FOR TOP BARS, CORNER BARS MAY BE OMITTED.
2. MATCH SIZE, LOCATION AND NUMBER OF HORIZONTAL BEAM BARS.

3 TYPICAL CORNER BAR REINFORCEMENT
N.T.S.



4 STEEL COLUMN FOOTING DETAIL
¾" = 1'-0"

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Details

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